

# EKSAMEN

Emnekode: IRB21512	Emnenavn: Konstruksjonsteknikk 1
Dato: 06.12.2018 Sensurfrist: 27.12.2018	Eksamenstid: 09.00-13.00
Antall oppgavesider: 4  Antall vedleggsider: 9	Faglærer: Bjarke Laustsen og Ole Bjerk  Oppgaven er kontrollert: Geir Flote
<b>Hjelpemidler:</b> NS-EN 1990, NS-EN 1991-1-1, NS-EN 1991-1-3, NS-EN 1991-1-4 uten notater. Utdelt kalkulator	
<b>Om eksamensoppgaven:</b> <i>Dersom du mener det mangler opplysninger: Gjør nødvendige antagelser og begrunn dette i besvarelsen.</i> <i>Vær kortfattet og bruk figurer.</i>	
Kandidaten må selv kontrollere at oppgavesettet er fullstendig	



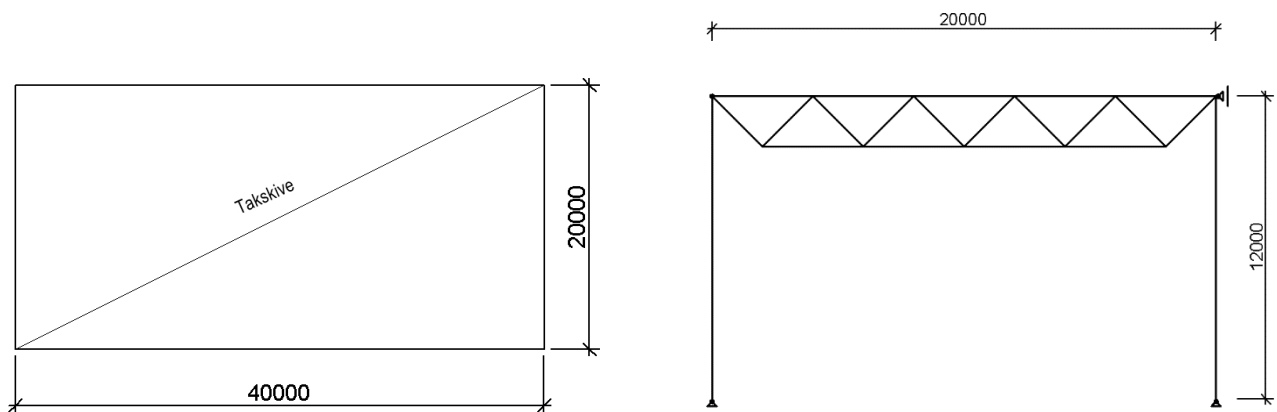
### Oppgave 1 - Jordskjelv, bære- og avstivningssystemer (5 %)

- Hvilke parametere bestemmer størrelsene på jordskjelvkraftene?
- Beskriv kort og presist begrepet "soft story".
- Beskriv hovedforskjellen på en skive og en plate. Forklar kort hvorfor vi ønsker å bruke skivevirkning til avstivning. Bruk gjerne figurer.
- Forklart kort og presist hvorfor avstivning i montasjefasen er viktig.

### Oppgave 2 - Laster og lastkombinasjoner (45 %)

Det vil bli oppgitt i oppgaven dersom laster skal kombineres. Dersom annet ikke er angitt skal karakteristiske laster benyttes (laster uten last- og kombinasjonsfaktor). Konstruksjonen er forenklet, dvs. at alle knutepunkt er leddet og det forutsettes statisk bestemt system for alle beregninger.

En idrettshall skal oppføres et sted i Lillehammer kommune i Oppland, 350 moh. Hallen har flatt tak. Rammeavstanden mellom de bærende rammene er 5 meter. Du skal se på en typisk midtramme i oppgavene under.



#### LASTER

Takskive:	0.6 kN/m <sup>2</sup>
Fagverksbjelke:	1.0 kN/m
Nyttelast:	1.0 kN/m <sup>2</sup>

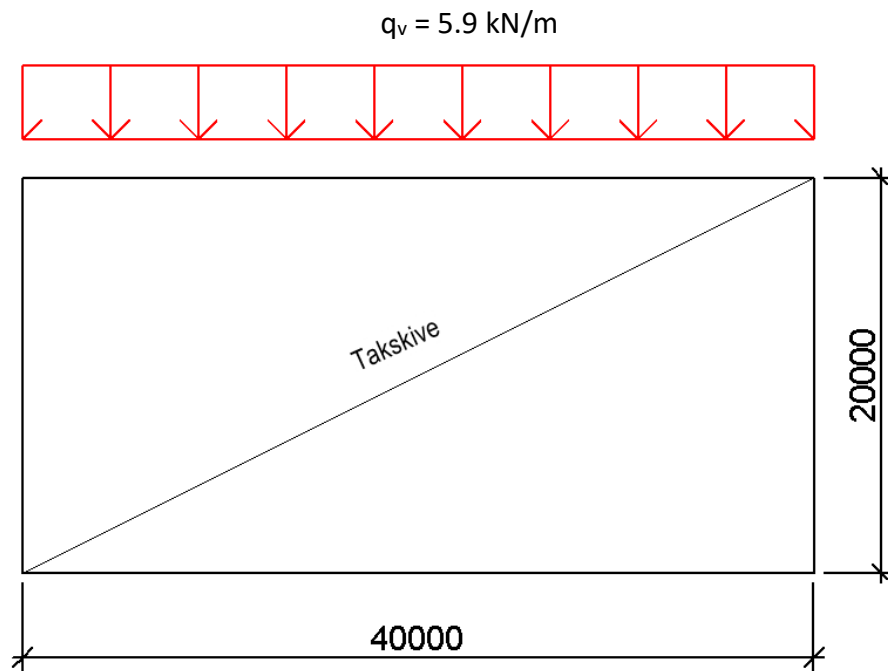
- Vis at snølasten på tak blir  $S = 4.4 \text{ kN/m}^2$ . Forutsett normal topografi og  $C_t = 1$
- Finn dimensjonerende last i **bruddgrensetilstand** som søylene skal dimensjoneres for. Medta alle aktuelle egenlast, snølast fra oppgave a) og oppgitt nyttelast.
- Lagerhallens grunnflate er 40x20 m (vist på figuren). Bygget har avstivende kryss i hver gavl (kortvegg). Forutsett terrengruhetskategori II og se bort ifra innvendig vindtrykk og vindlast på tak.

Vis at vindlasten (sone D og E) som virker på **søylene** rammen er 3,0 kN/m og 1.9 kN/m når veggpanelene spenner fra søyle til søyle. Benytt  $C_{pe,10}$

Bygget benytter takskiven for å fordele vindkrefter til yttervegg:

- d) Tegn opp statisk modell, med tilhørende moment- og skjærkraftdiagram. Regn ut og angi maksimalt moment og skjærkraft i **bruddgrensetilstand** (kun vind). Finn randstrekket i kantbjelken på byggets langside.

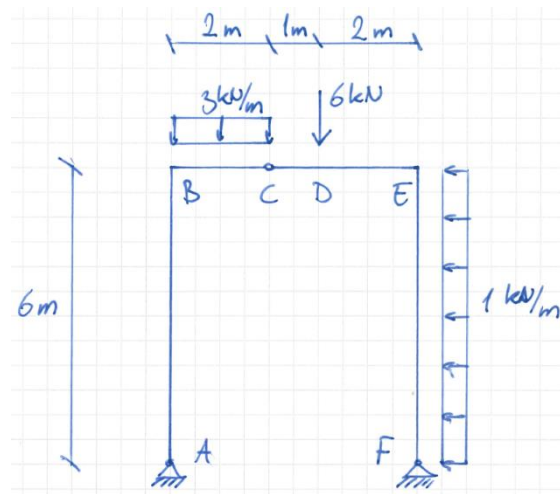
Forutsett  $q_v = 5.9 \text{ kN/m}$  (karakteristisk last).



- e) Regn ut flatelast [ $\text{kN/m}^2$ ] for sone F på tak når vinden blåser på langveggen. Anta skarp takavslutning. Regn ut arealet av sone F. Benytt  $C_{pe,1}$

### Oppgave 3 - Ramme (20 %)

Gitt en ramme med viste laster og randbetingelser.

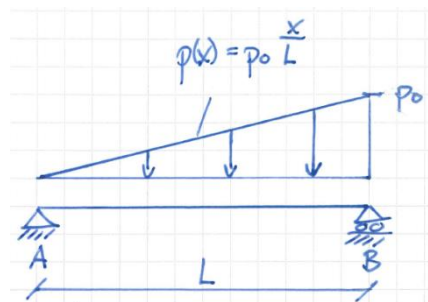


- Hvorfor er konstruksjonen statisk bestemt?
- Bestem reaksjonskrefter.
- Tegn snittkraftdiagrammer (M, V og N) og angi verdier.

### Oppgave 4 – Bernoullis bjelke (15 %)

Gitt en bjelke med viste last og randbetingelser.

$EI = \text{konstant}$

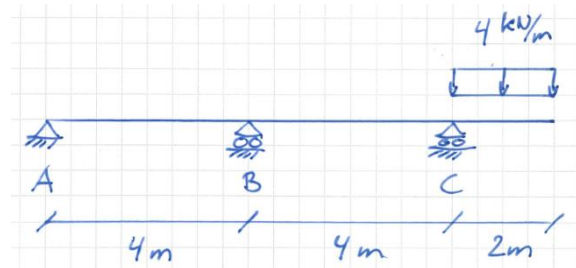


- Angi geometriske og statiske randbetingelser.
- Bestem bjelkens utbøyningskurve.
- Bestem bjelkens helling i A.
- Finn punktet med maksimum moment.

### Oppgave 5 – Statisk ubestemt bjelke (15 %)

Gitt en kontinuerlig bjelke med viste last og randbetingelser.

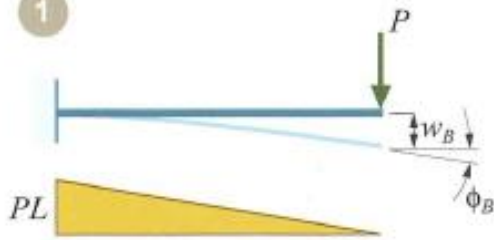
$EI = \text{konstant}$



- Vis at bjelken er 1 gang statistisk ubestemt.
- Bestem reaksjonskrefter.
- Tegn momentdiagrammet og angi verdier.

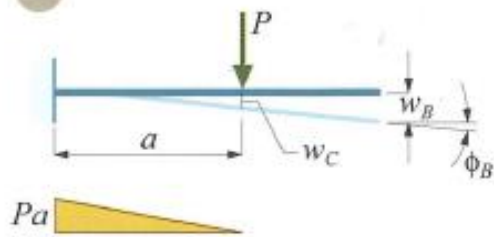
Vedlegg – bjelkeformler

1



$$w_B = \frac{PL^3}{3EI} \quad \phi_B = \frac{PL^2}{2EI}$$

2



$$w_C = \frac{Pa^3}{3EI} \quad w_B = \frac{Pa^2}{6EI}(3L - a)$$

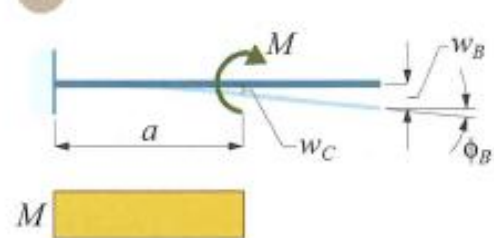
$$\phi_B = \frac{Pa^2}{2EI}$$

3



$$w_B = \frac{ML^2}{2EI} \quad \phi_B = \frac{ML}{EI}$$

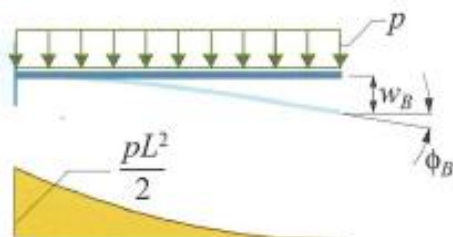
4



$$w_C = \frac{Ma^2}{2EI} \quad w_B = \frac{Ma}{2EI}(2L - a)$$

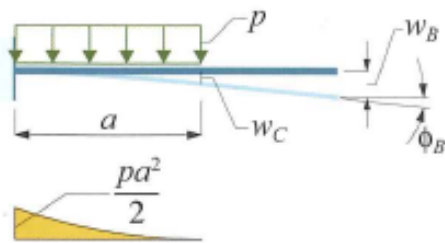
$$\phi_B = \frac{Ma}{EI}$$

5



$$w_B = \frac{pL^4}{8EI} \quad \phi_B = \frac{pL^3}{6EI}$$

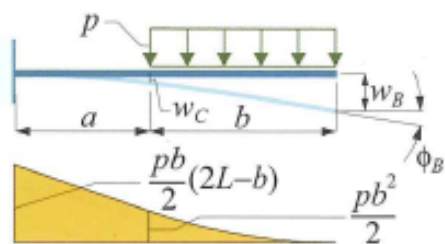
6



$$w_C = \frac{pa^4}{8EI} \quad w_B = \frac{pa^3}{24EI}(4L - a)$$

$$\phi_B = \frac{pa^3}{6EI}$$

7

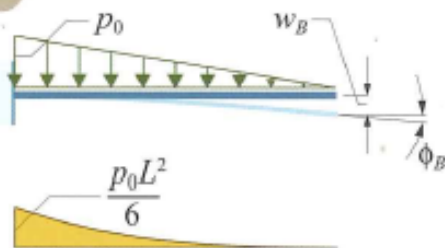


$$w_C = \frac{pa^2b}{12EI}(3L + a)$$

$$w_B = \frac{p}{24EI}(3L^4 - 4a^3L + a^4)$$

$$\phi_B = \frac{p}{6EI}(L^3 - a^3)$$

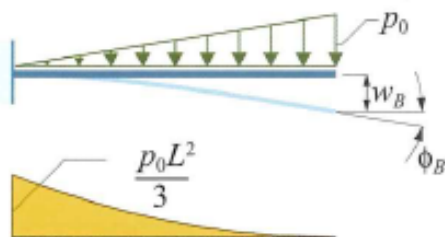
8



$$w_B = \frac{p_0L^4}{30EI}$$

$$\phi_B = \frac{p_0L^3}{24EI}$$

9



$$w_B = \frac{11p_0L^4}{120EI}$$

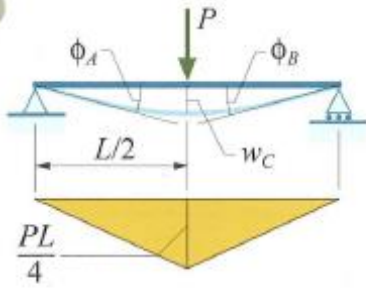
$$\phi_B = \frac{p_0L^3}{8EI}$$

10



$$u_B = \frac{PL}{EA}$$

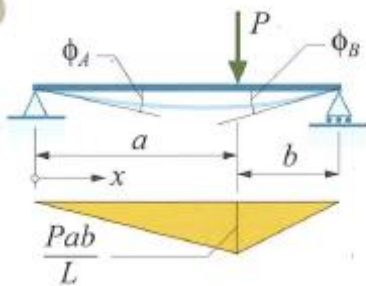
11



$$\phi_A = \phi_B = \frac{PL^2}{16EI}$$

$$w_C = \frac{PL^3}{48EI}$$

12

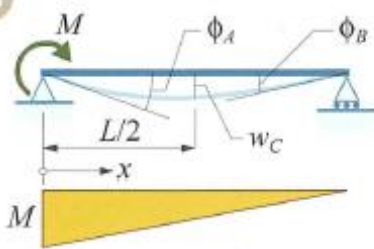


$$\phi_A = \frac{Pab(L+b)}{6L \cdot EI}$$

$$\phi_B = \frac{Pab(L+a)}{6L \cdot EI}$$

$$w(x) = \frac{Pbx}{6L \cdot EI}(L^2 - b^2 - x^2), \quad x \leq a$$

13

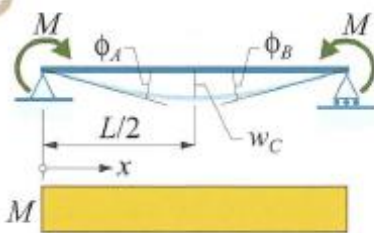


$$\phi_A = \frac{ML}{3EI} \quad \phi_B = \frac{\phi_A}{2} = \frac{ML}{6EI}$$

$$w(x) = \frac{M(L-x)}{6L \cdot EI}(2Lx - x^2)$$

$$w_C = \frac{ML^2}{16EI}$$

14

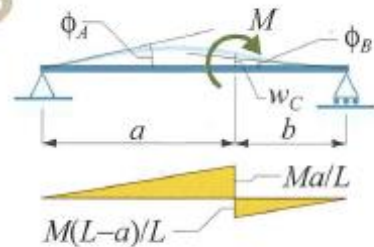


$$\phi_A = \phi_B = \frac{ML}{2EI}$$

$$w(x) = \frac{Mx}{2EI}(L-x)$$

$$w_C = \frac{ML^2}{8EI}$$

15



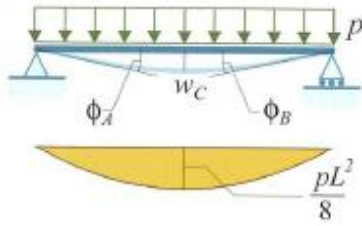
$$\phi_A = \frac{M}{6L \cdot EI}(6aL - 3a^2 - 2L^2)$$

$$\phi_B = \frac{M}{6L \cdot EI}(3a^2 - L^2)$$

$$w_C = \frac{Mab}{3L \cdot EI}(2a - L)$$



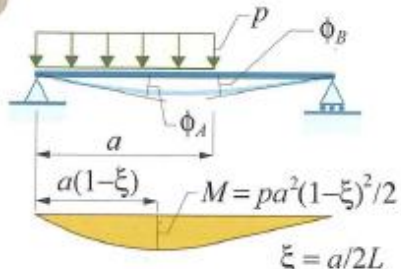
16



$$\phi_A = \phi_B = \frac{pL^3}{24EI}$$

$$w_C = \frac{5pL^4}{384EI}$$

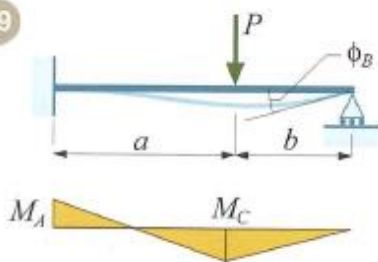
17



$$\phi_A = \frac{pa^2}{24L \cdot EI} (2L - a)^2$$

$$\phi_B = \frac{pa^2}{24L \cdot EI} (2L^2 - a^2)$$

19

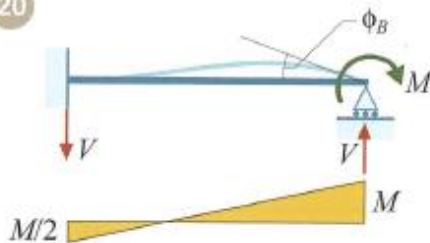


$$\phi_B = \frac{Pa^2b}{4L \cdot EI}$$

$$M_A = \frac{Pab}{2L} \left(1 + \frac{b}{L}\right)$$

$$M_C = \frac{Pab}{2L} \left[2 - \frac{b}{L} - \left(\frac{b}{L}\right)^2\right]$$

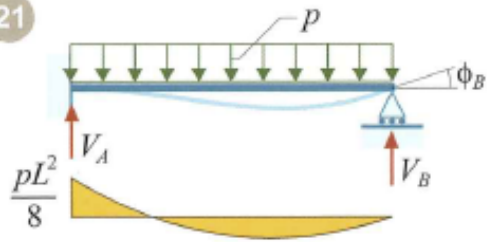
20



$$\phi_B = \frac{ML}{4EI}$$

$$V = \frac{3M}{2L}$$

21

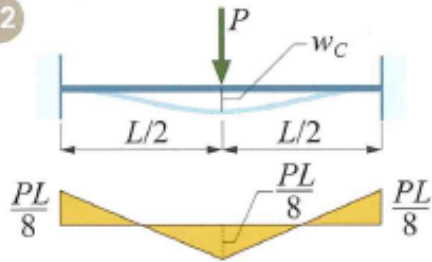


$$\phi_B = \frac{pL^3}{48EI}$$

$$V_A = \frac{5pL}{8}$$

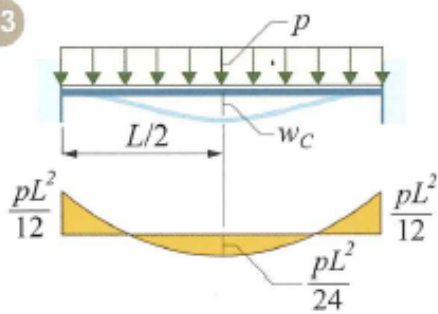
$$V_B = \frac{3pL}{8}$$

22



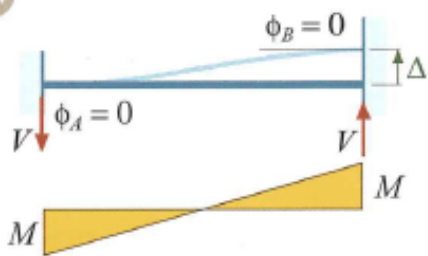
$$w_c = \frac{PL^3}{192EI}$$

23



$$w_c = \frac{pL^4}{384EI}$$

24

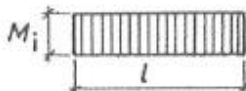

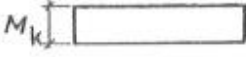




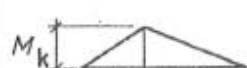
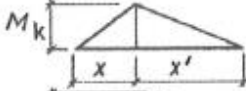
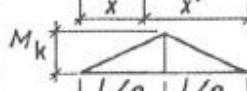
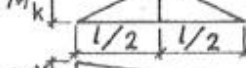


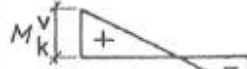
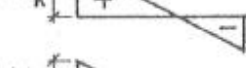

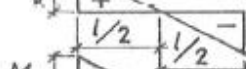
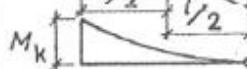
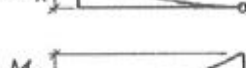


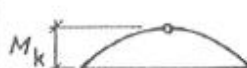
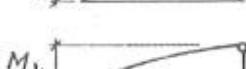

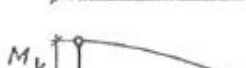



$$M = \frac{6EI}{L^2} \Delta$$

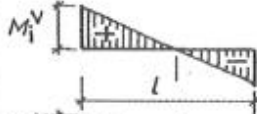

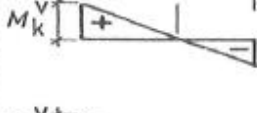
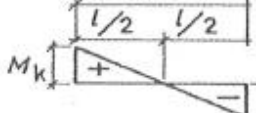

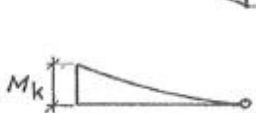
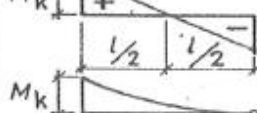

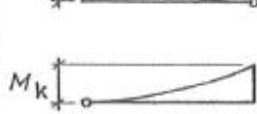

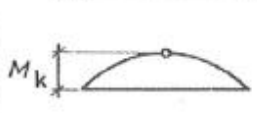
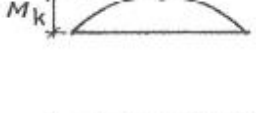
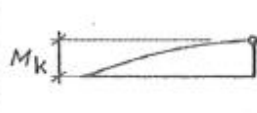

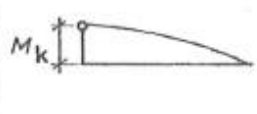
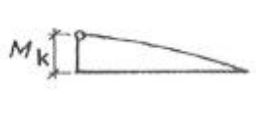


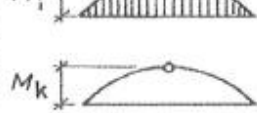

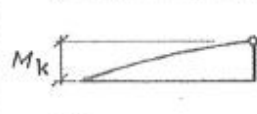
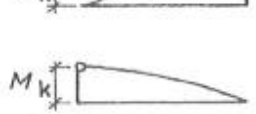
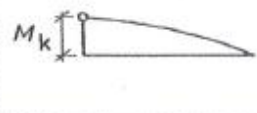

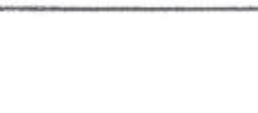

$$V = \frac{12EI}{L^3} \Delta$$

## VEDLEGG: Integrasjonstabeller

$$EI \cdot \delta_{iK} = \int_0^l M_i \cdot M_K \cdot dx \quad \text{og} \quad EI \cdot \delta_{ii} = \int_0^l M_i^2 \cdot dx.$$

	$M_i^2 l$		$\frac{1}{3} M_i^2 l$
	$M_i M_K l$		$\frac{1}{3} M_i M_K l$
	$\frac{1}{2} M_i M_K l$		$\frac{1}{8} M_i M_K l$
	$\frac{1}{2} M_i M_K l$		$\frac{1}{6} M_i M_K l \left(1 + \frac{x'}{l}\right)$
	$\frac{1}{2} M_i M_K l$		$\frac{1}{4} M_i M_K l$
	$\frac{1}{2} M_i M_K l$		$\frac{1}{6} M_i (2M_K^V + M_K^H) l$
	$\frac{1}{2} M_i (M_K^V + M_K^H) l$		$\frac{1}{6} M_i (2M_K^V - M_K^H) l$
	$\frac{1}{2} M_i (M_K^V - M_K^H) l$		$\frac{1}{6} M_i M_K l$
	0		$\frac{1}{4} M_i M_K l$
	$\frac{1}{3} M_i M_K l$		$\frac{1}{12} M_i M_K l$
	$\frac{1}{3} M_i M_K l$		$\frac{1}{3} M_i M_K l$
	$\frac{2}{3} M_i M_K l$		$\frac{1}{4} M_i M_K l$
	$\frac{2}{3} M_i M_K l$		$\frac{5}{12} M_i M_K l$

NB! Parablene har sine toppunkter (horisontal tangent) i de punkter som er markert med en ring på figurene.

 $M_i^H \frac{1}{3} (M_i^H - M_i^V M_i^H + M_i^H^2) l$	 $\frac{1}{3} M_i^2 \cdot l$
 $\frac{1}{6} [2(M_i^V M_k^V + M_i^H M_k^H) - M_i^H M_k^V - M_i^V M_k^H] l$	 $\frac{1}{3} M_i M_k l$
	 $\frac{1}{6} M_i M_k l$
 $\frac{1}{6} M_k (M_i^V + M_i^H) l$	 $-\frac{1}{6} M_i M_k l$
 $\frac{1}{12} M_k (3M_i^V - M_i^H) l$	 $-\frac{1}{6} M_i M_k l$
 $\frac{1}{12} M_k (M_i^V - 3M_i^H) l$	 $0$
 $\frac{1}{3} M_k (M_i^V - M_i^H) l$	 $-\frac{1}{6} M_i M_k l$
 $\frac{1}{12} M_k (3M_i^V - 5M_i^H) l$	 $\frac{1}{6} M_i M_k l$
 $\frac{1}{12} M_k (5M_i^V - 3M_i^H) l$	 $\frac{1}{6} M_i M_k l$
 $\frac{8}{15} \cdot M_i^2 \cdot l$	 $\frac{8}{15} M_i^2 l$
 $\frac{8}{15} \cdot M_i \cdot M_k l$	 $\frac{8}{15} M_i M_k l$
 $\frac{7}{15} \cdot M_i \cdot M_k l$	 $\frac{11}{30} M_i M_k l$
 $\frac{7}{15} \cdot M_i \cdot M_k l$	 $\frac{7}{15} M_i M_k l$

	$\frac{1}{3} M_i^2 l$		$\frac{1}{3} M_i^2 l$
	$\frac{1}{3} M_i M_k l \left( \frac{l}{2} - \frac{2x^2}{3l} \right)$		$\frac{1}{3} M_i M_k l$
	$\frac{1}{3} M_i M_k \cdot l$		$\frac{1}{4} M_i (M_k^V + M_k^H) l$
	$\frac{1}{6} M_i M_k l \left( 2 - \frac{\bar{x}^2}{x_1 \cdot x_1'} \right)$		$\frac{1}{4} M_i (M_k^V - M_k^H) l$
	$\frac{1}{6} M_i \left[ M_k^V \left( 1 + \frac{x'}{l} \right) + M_k^H \left( 1 + \frac{x}{l} \right) \right] l$		0
	$\frac{1}{6} M_i \left[ M_k^V \left( 1 + \frac{x'}{l} \right) - M_k^H \left( 1 + \frac{x}{l} \right) \right] l$		$\frac{7}{48} M_i M_k l$
	$\frac{1}{3} M_i M_k l \cdot \frac{\bar{x}}{l}$		$\frac{7}{48} M_i M_k l$
	$\frac{1}{12} M_i M_k l \left( \frac{3x'}{l} + \frac{x^2}{l^2} \right)$		$\frac{5}{12} M_i M_k l$
	$\frac{1}{12} M_i M_k l \left( \frac{3x}{l} + \frac{x'^2}{l^2} \right)$		$\frac{17}{48} M_i M_k l$
	$\frac{1}{3} M_i M_k l \left( 1 + \frac{x \cdot x'}{l^2} \right)$		$\frac{17}{48} M_i M_k l$
	$\frac{1}{12} M_i M_k l \left( 3 + \frac{3x}{l} - \frac{x^2}{l^2} \right)$		
	$\frac{1}{12} M_i M_k l \left( 3 + \frac{3x'}{l} - \frac{x'^2}{l^2} \right)$		

